

A simple and rapid approach for the numerical simulation of non-linear elements and examples of its application

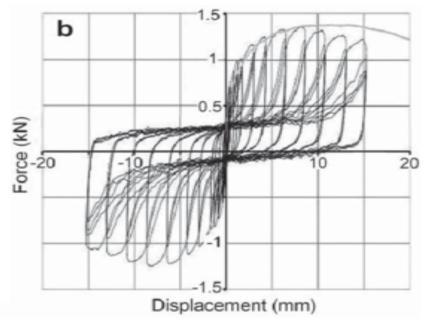
W Loo and L Tuleasca

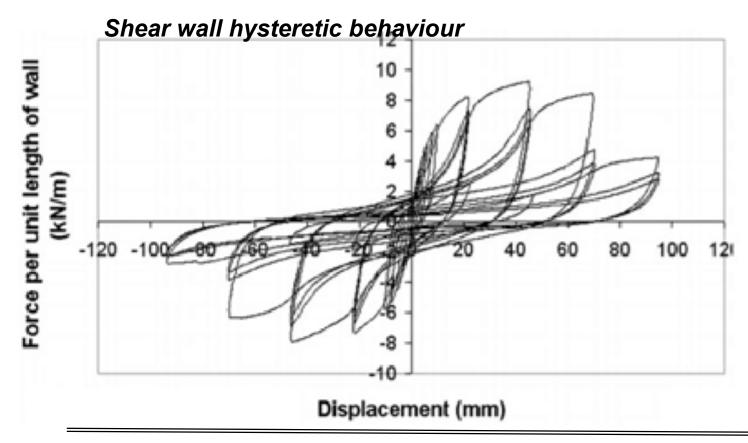
UNITEC Institute of Technology – Department of Civil Engineering, 139 Carrington Road, Mt Albert, Auckland, New Zealand



Pinched hysteretic behavior of nails reflected in hysteretic behavior of shear walls

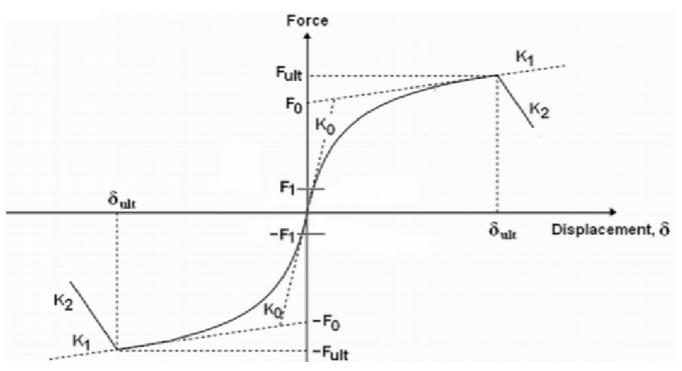
Typical nail hysteretic behaviour







Backbone curve of fastener through wood



The benchmark force-displacement relationship for modelling nails, will take the form of the well-known Foschi exponential curve (Dolan and Madsen 1992). Eq. (1) describes the curve between zero and ultimate displacement, δ_{ult}

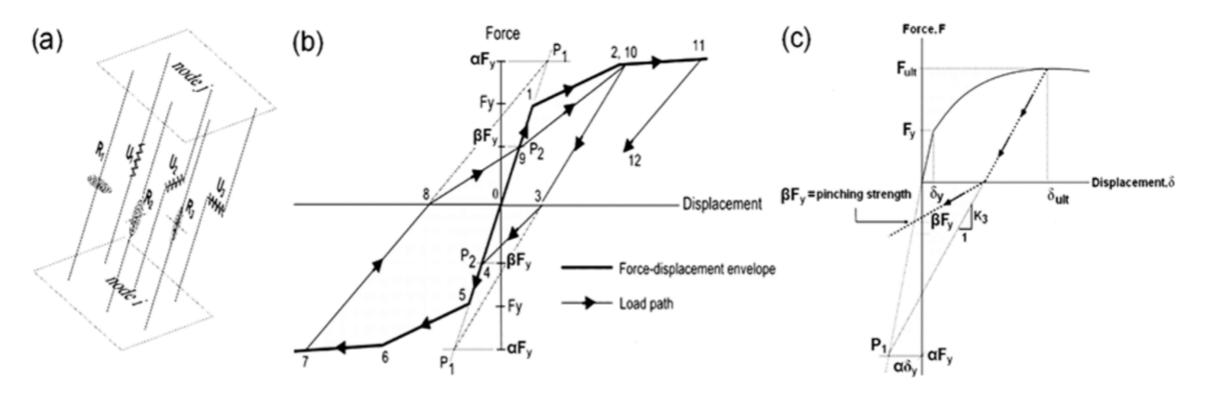
$$Force = (F_0 + K_1 \delta) \cdot [1 - \exp(-K_0 \delta / F_0)] \tag{1}$$

and Eq. (2) describes the relationship for displacements beyond δ_{ult}

$$Force = K_2 \delta + (F_{ult} - K_2 \delta_{ult}) \tag{2}$$



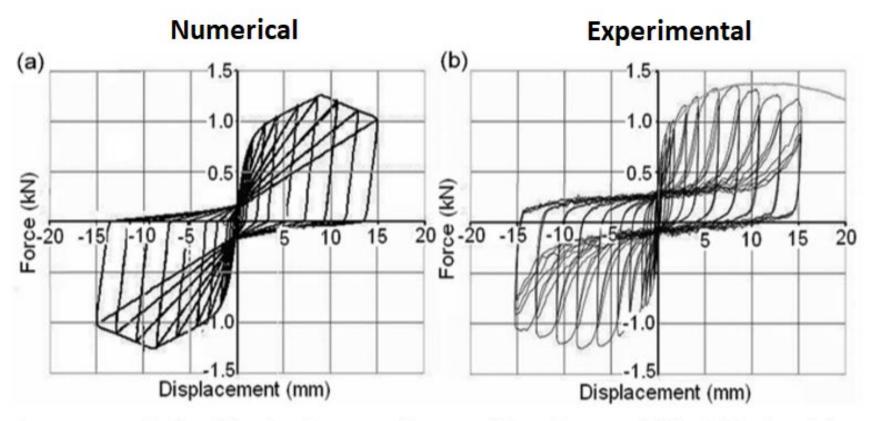
Hysteretic behavior modelled by single link element



Nail connection (a) multilinear plastic link element adopted consists of translational and rotational springs (b) multi-linear plastic link -'pivot' hysteresis type, and (c) determination of hysteresis parameters α , and β



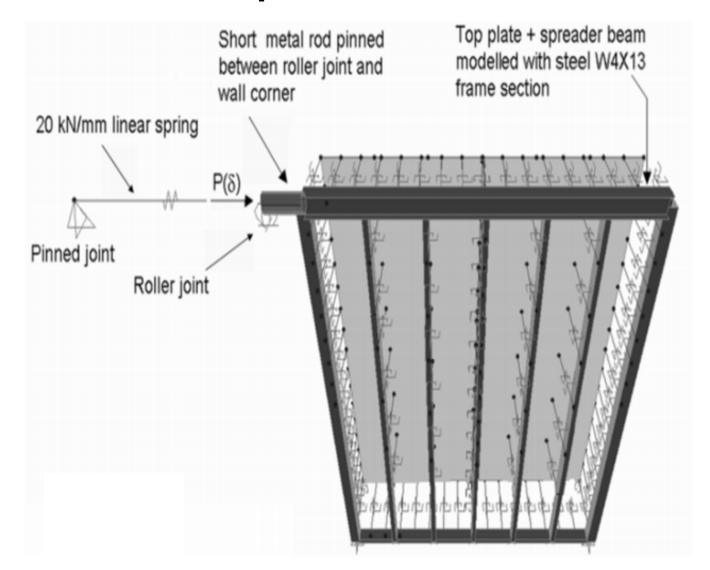
Hysteretic behavior modelled by single link element



Force-displacement relationship for 3 mm nails attaching 11 mm thick OSB sheathing to SPF framing (a) Numerical simulation and (b) experimental result (courtesy of Dinehart et al. (2006))

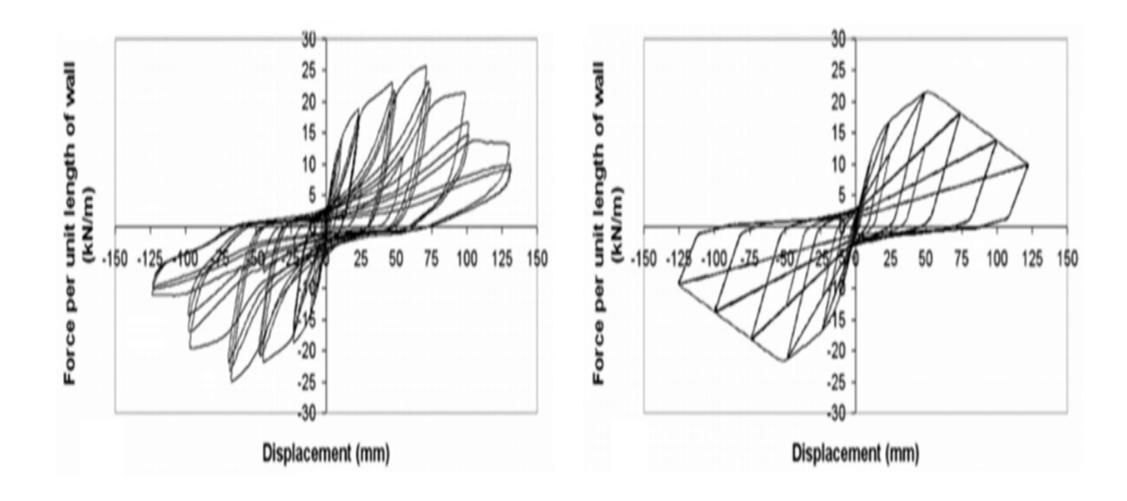


Elements are implemented in walls



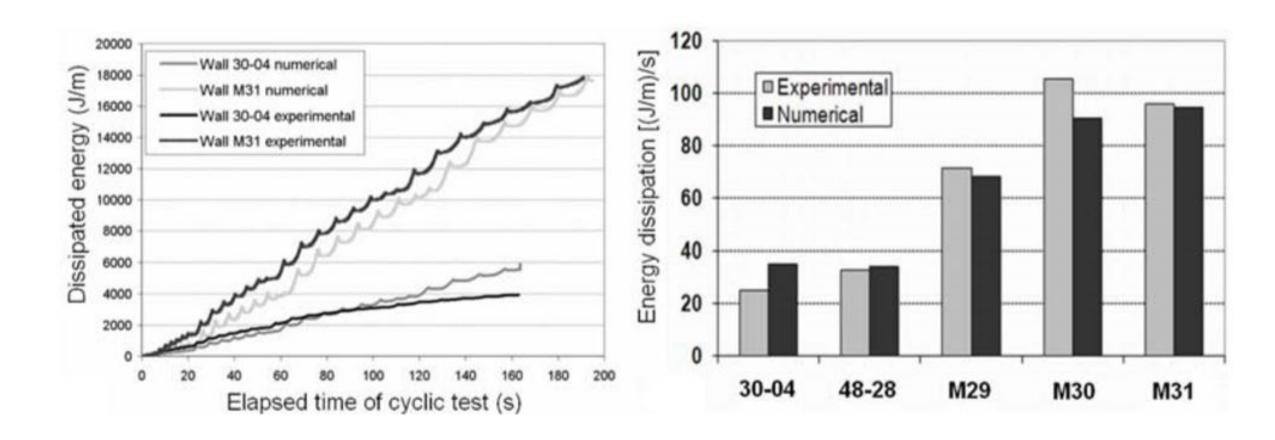


Good match between numerical and experimental





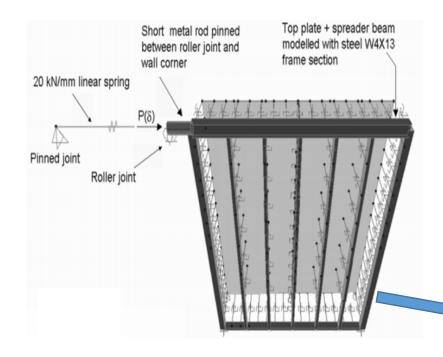
Energy dissipation comparison

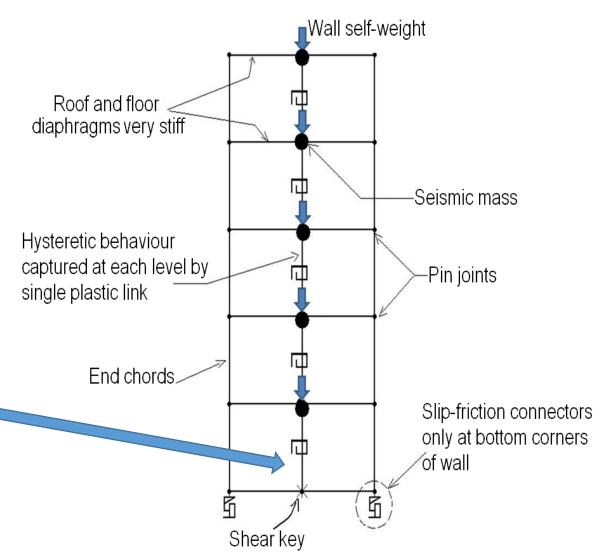




The behavior of a wall with many links can be captured by

a single link

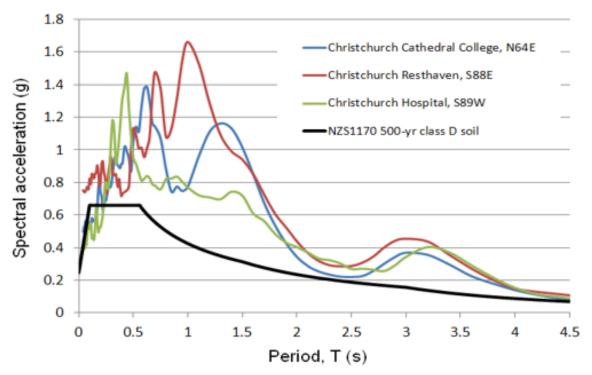






Christchurch, Feb 2011

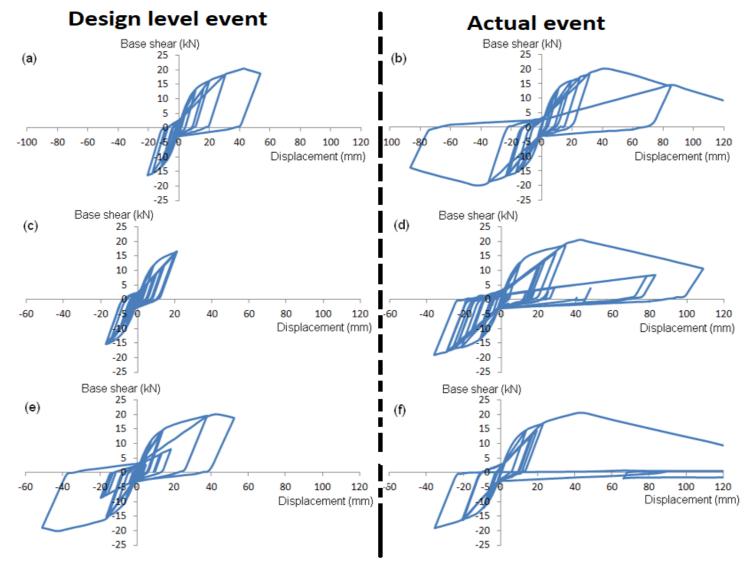
The authors modelled a shear wall using the methodology of the preceding section and subjected it to a sequence of simulated earthquake loadings. Earthquake motions from the destructive 22nd February 2011 Christchurch earthquake (magnitude 6.3 M₁) were applied. The data used were from three different sites in the Christchurch central business district: Christchurch Cathedral College, Christchurch Hospital, and Christchurch Resthaven (Note Zone factor for Christchurch was 0.22, is now 0.30



Spectral accelerations for the February 2011 Christchurch earthquake at three sites, and the ULS design spectrum (500 yr return period) for NZS1170.5 [16], Type D (soft) soils (Spectra produced from data provided by GeoNet NZ [17])

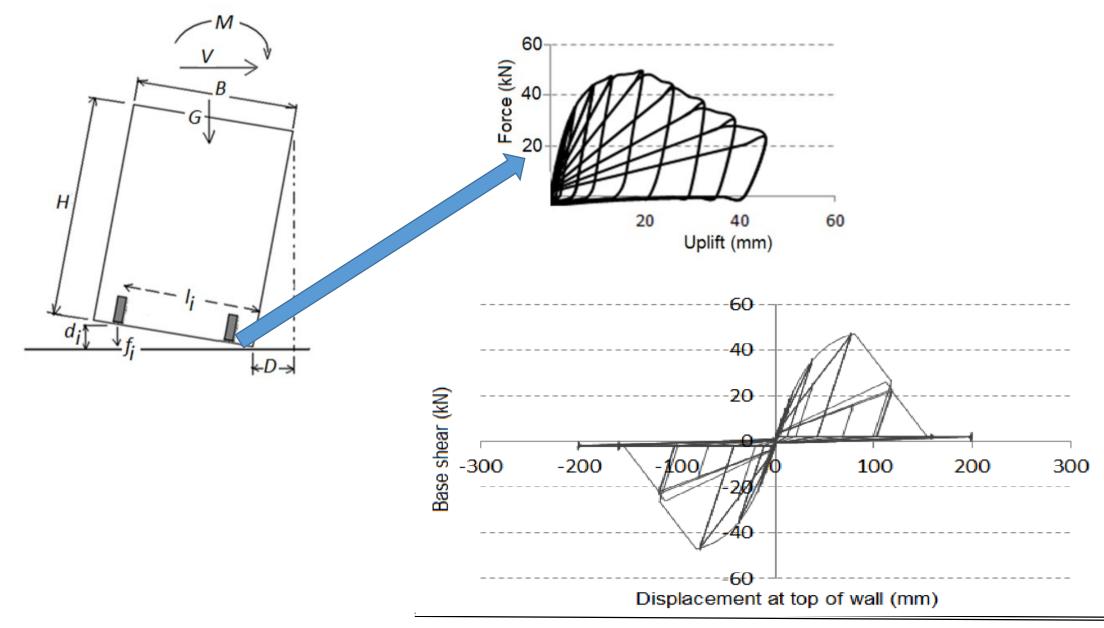


Christchurch: Numerical comparison between actual and Design level (previous) events, single storey wall





Other applications: Modelled with CLT wall panels

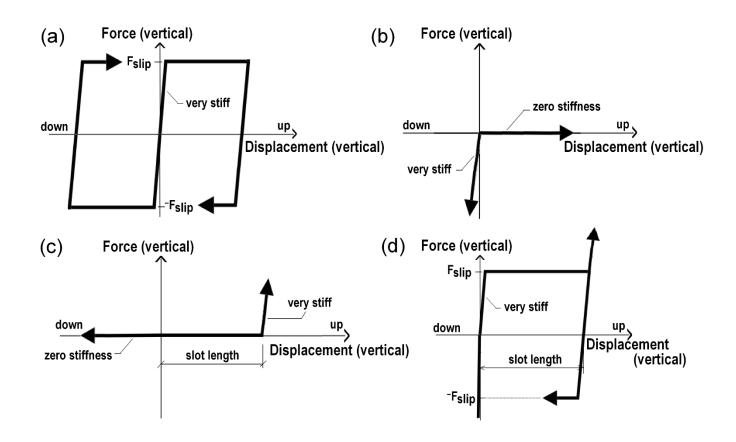




Other applications:

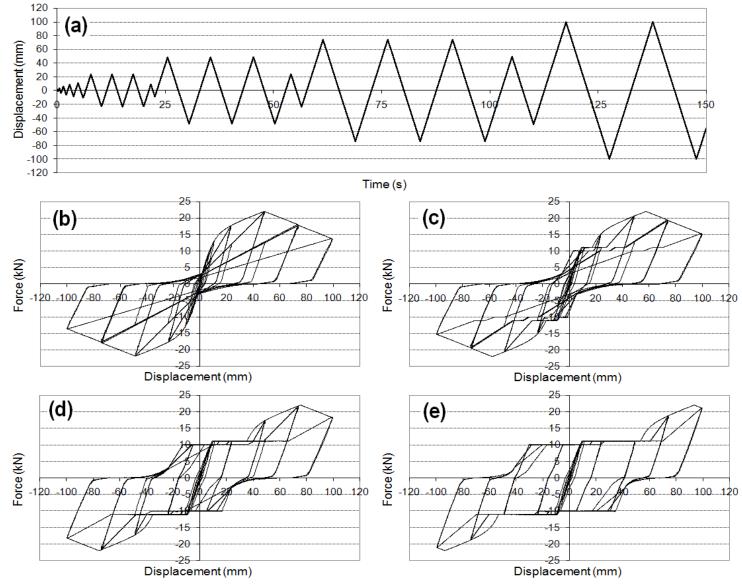
Combined with passive energy friction dissipaters modelled numerically:



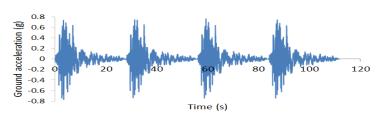




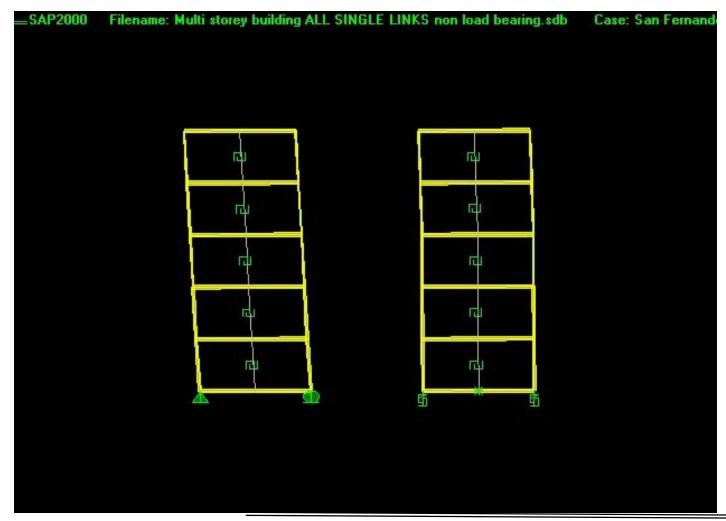
Other applications: Combined with passive energy friction dissipaters modelled numerically:







San Fernando scaled events, 4 in succession, scale factor x 5





Conclusion

 Quick and effective way to model non-linear behavior

Suitable for use by consultants

 Parametric studies can be carried out with automated variation of parameters running of load cases through the use of VBA programming accessing the API of a finite element software package.

